GLENN COUNTY HAMILTON CITY PARK ACCESSIBILITY IMPROVEMENTS

ABBREVIATIONS

	ASPHALT BASE	FSR	FIRE SPRINKLER	PR	PAIR	UBC	UNI
	ACCESSIBLE		RISER	PT	PAINT OR POINT		C
	AREA DRAIN AMERICANS WITH	FD	FLOOR DRAIN	PL PREFIN	PROPERTY LINE PREFINISHED	UON	UNI O
	DISABLITIES ACT ADJACENT OR	GA GAL	GAGE OR GAUGE GALLON	PROP	PROPERTY		Ν
	ADJUSTABLE	GALV	HOT DIPPED	RWL	RAIN WATER	VTR	VE
	ADMINISTRATION ABOVE FINISH FLOOR	GI	GALVINIZED GALVANIZED IRON	RWD	LEADER REDWOOD	VIF	R(VEF
	AGGREGATE	GC	GENERAL	REINF	REINFORCED	VERT	VEF
	ALUMINUM APPROXIMATE		CONTRACTOR	R RD	RADIUS ROOF DRAIN	VWF	VIN
	ARCHITECTURAL	HT	HEIGHT	RO	ROUGH OPENING	WT	WE
	AMERICAN NATIONAL STANDARS	HCWD	HOLLOW CORE WOOD DOOR	RM	ROOM	W W/	WE WIT
	INSTITUTE ASPHALT	HM HMD	HOLLOW METAL HOLLOW METAL	SG	SAFETY GLASS	W/O	WIT
	ASPHALT AUDIO/VISUAL		DOOR	SCHED SDST	SCHEDULE SELF DRILLING	WI WSBC	WO WA
	BOARD	HORIZ	HORIZONTAL	SHTG	SELF TAPPING SHEATHING		ST C(
	BLOCKING	HORIZ	HORIZONTAL	SHT	SHEET		
	BASIS OF DESIGN BOTTOM	HB HD	HOSE BIB HOT DIPPED	SMF	SHEET METAL FASTENER		
	CENTER			SMS	SHEET METAL		
	CENTER LINE	ICC	INTERNATIONAL CODE COUNCIL	SIM	SCREW SIMILAR		
	CLEAR COLUMN	INC ID	INCLUDING INSIDE DIAMETER	STC	SOUND TRANSMISSION		
	CONCRETE	INSUL	INSULATION		CLASS		
	CONCRETE MASONRY UNIT	INT ISA	INTERIOR INTERNATIONAL	S SPEC	SOUTH SPECIFICATION		
	CONTINUOUS		SYMBOL OF	SQ	SQUARE		
	CONTROL JOINT CORRIDOR		ACCESSIBILITY	SF SCD	SQUARE FEET SEE CIVIL		
	CONTRACTOR CONTRACTOR	JAN	JANITOR		DRAWINGS		
	UNITAUIUR	MFR	MANUFACTURER	SSD	SEE STRUCTURAL DRAWINGS		
	DECOMPOSED	MAX MECH	MAXIMUM MECHANICAL	SST STD	STAINLESS STEEL STANDARD		
	GRANITE	MTL	METAL	SFM	STATE FIRE		
	DEGREES DEMOLITION	MIN MISC	MINIMUM MISCELLANEOUS	STL	MARSHAL STEEL		
	DETAIL DIAMETER	(N)	NEW	STOR STRUCT	STORAGE STRUCTURAL		
	DIVISION	ŇÓM	NOMINAL	SUSP	SUSPENDED		
	DOOR DOUBLE	N NIC	NORTH NOT IN CONTRACT	SYS	SYSTEM		
	DOWNSPOUT	NTS	NOT TO SCALE	TB T º P			
	EACH	0/	OVER	T&B TBR	TOP AND BOTTOM TO BE REMOVED		
	EAST ELECTRICAL	OFF OC	OFFICE ON CENTER	THK THRES	THICKNESS THRESHOLD		
	EQUAL	ОН	OPPOSITE HAND	THRU	THROUGH		
	EQUIPMENT EXISTING TO	OD OFCI	OUTSIDE DIAMETER OWNER FURNISHED	TO TOB	TOP OF TOP OF BEAM		
	RECEIVE IMPROVEMENTS	-	CONTRACTOR	TOC	TOP OF CONCRETE	\$	
E)	EXISTING	OFOI	INSTALLED OWNER FURNISHED	TOS TPO	TOP OF SLAB THERMOPLASTIC		
	EXTERIOR		OWNER INSTALLED	TRTD	POLYOLEFIN TREATED		
	FINISH FLOOR	CFCI	CONTRACTOR	TS	TOP OF SLAB OR		
	FIRE EXTINGUISHER		FURNISHED CONTRACTOR	TYP	TUBE STEEL TYPICAL		
	CABINET		INSTALLED				
			SYMBOL		חו		
						•	
\neq			A			A	
			\checkmark			<u> </u>	
\langle						SIGNAGE INDIC	<u>ATOR</u>
			GRID LINE				_
<u>1</u>	NORTH INDICATOR			DETAIL NUMBER		+7 8	.00
/	ELEV. NUMBER			NUMBER			
A	<u>1</u> -201A		(1) (A-560)	SHT. NUMBER			
	SHT. NUMBER					VERTICAL DIMEN	NJION P
BU	ILDING ELEVATION		(1) (A-560))			
	INDICATOR		A-300			<u>(</u>)−− 0.0	00
			DETAIL INDICATO	<u>RS</u>		\cup 0.0	-
/	SECTION		ELEVATI	ON		DATUM PO	<u>INT</u>
	A1 -301A			R			
7	SHT. NUMBER					(101A)	
в	UILDING SECTION				_		
	INDICATOR		SHEET	NUMBER	<u>]</u>	<u>DOOR OPENING I</u>	NDICAT

N

INTERIOR ELEVATION

0

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FF FE FEC

DEMO

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APPROX

AFF

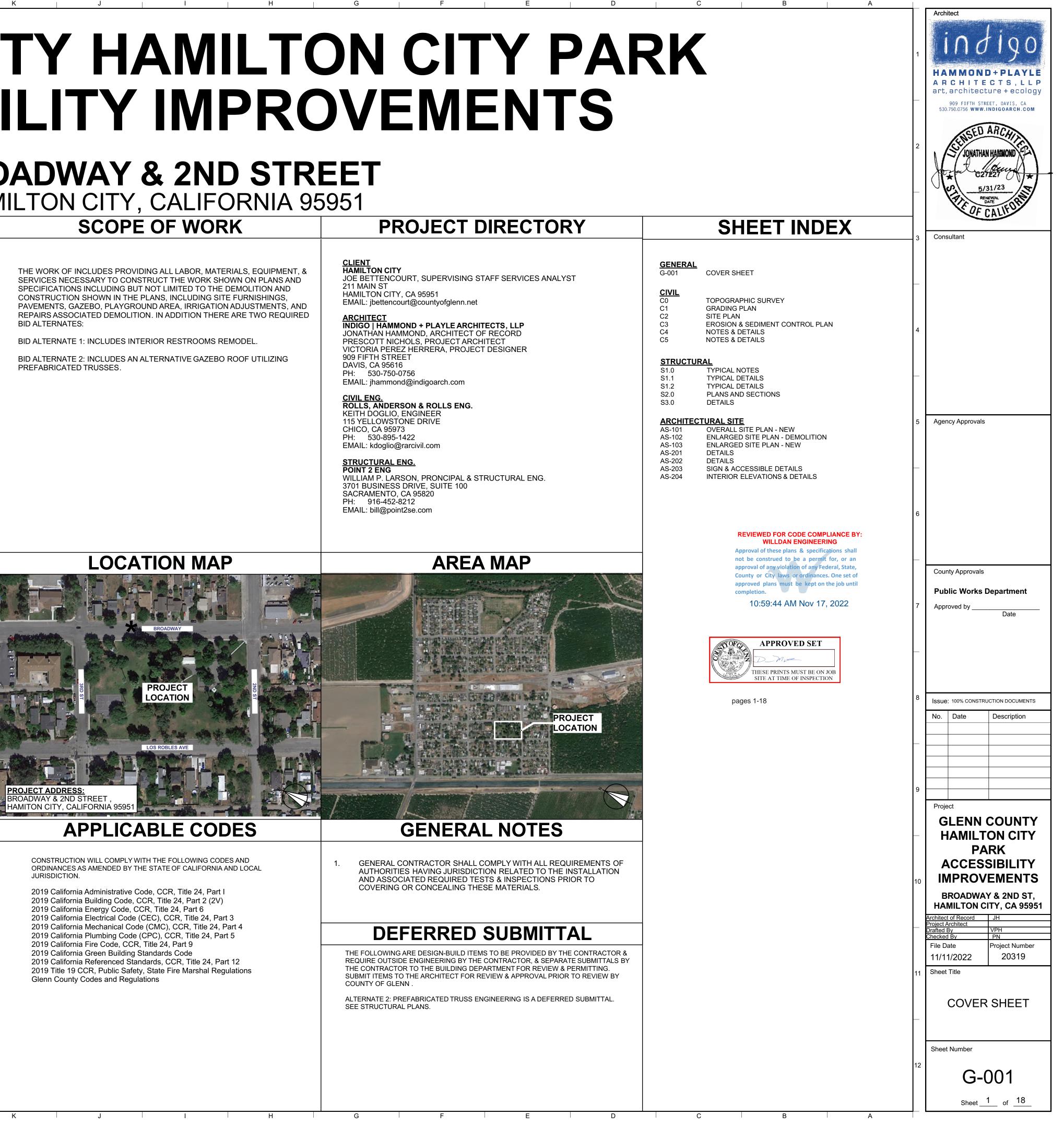
DOOR OPENING INDICATOR

BROADWAY & 2ND STREET HAMILTON CITY, CALIFORNIA 95951

UNIFORM BUILDING CODE UNLESS OTHERWISE NOTED

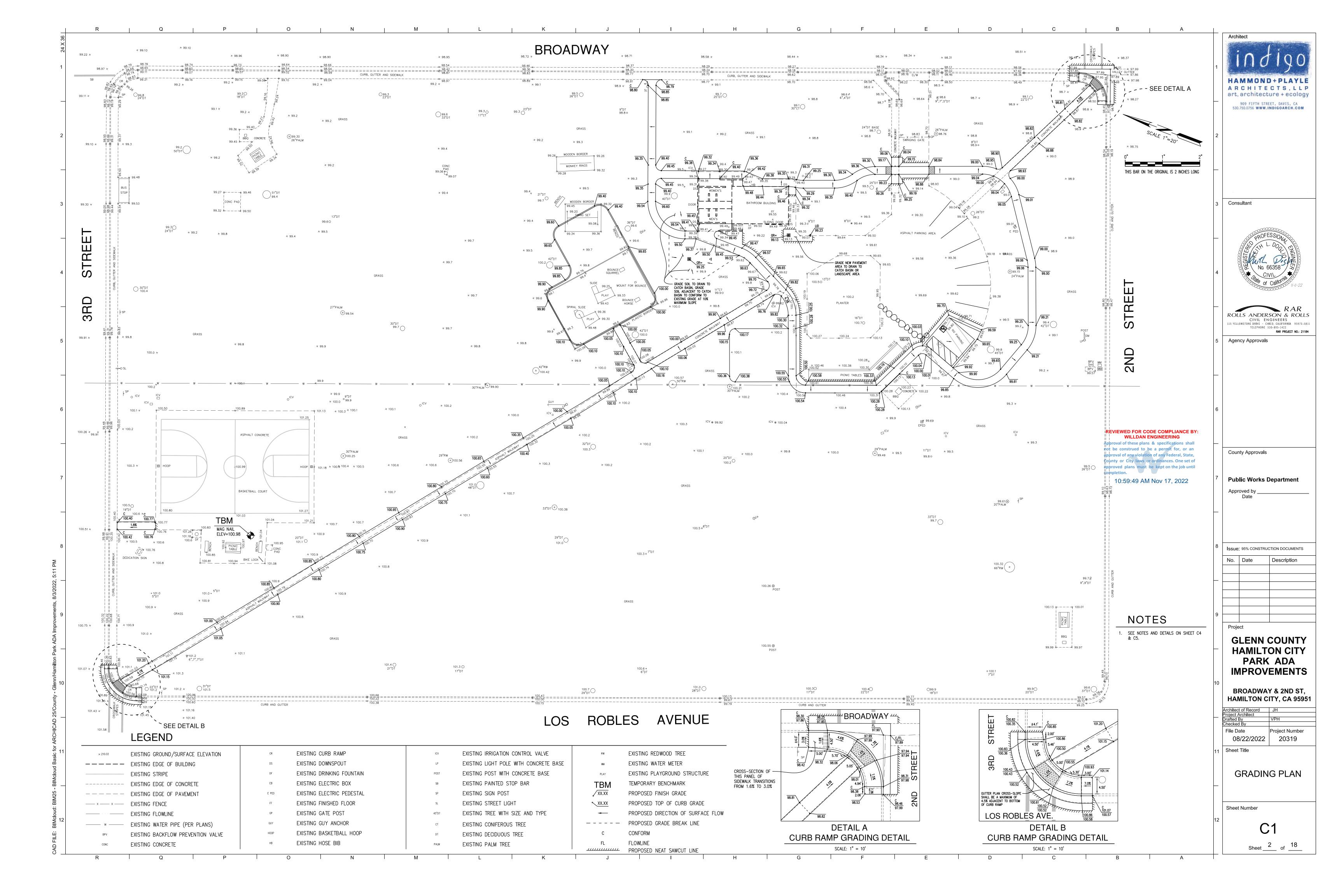
VENT THROUGH ROOF **VERIFY IN FEILD** VERTICAL VINYL WALL FABRIC

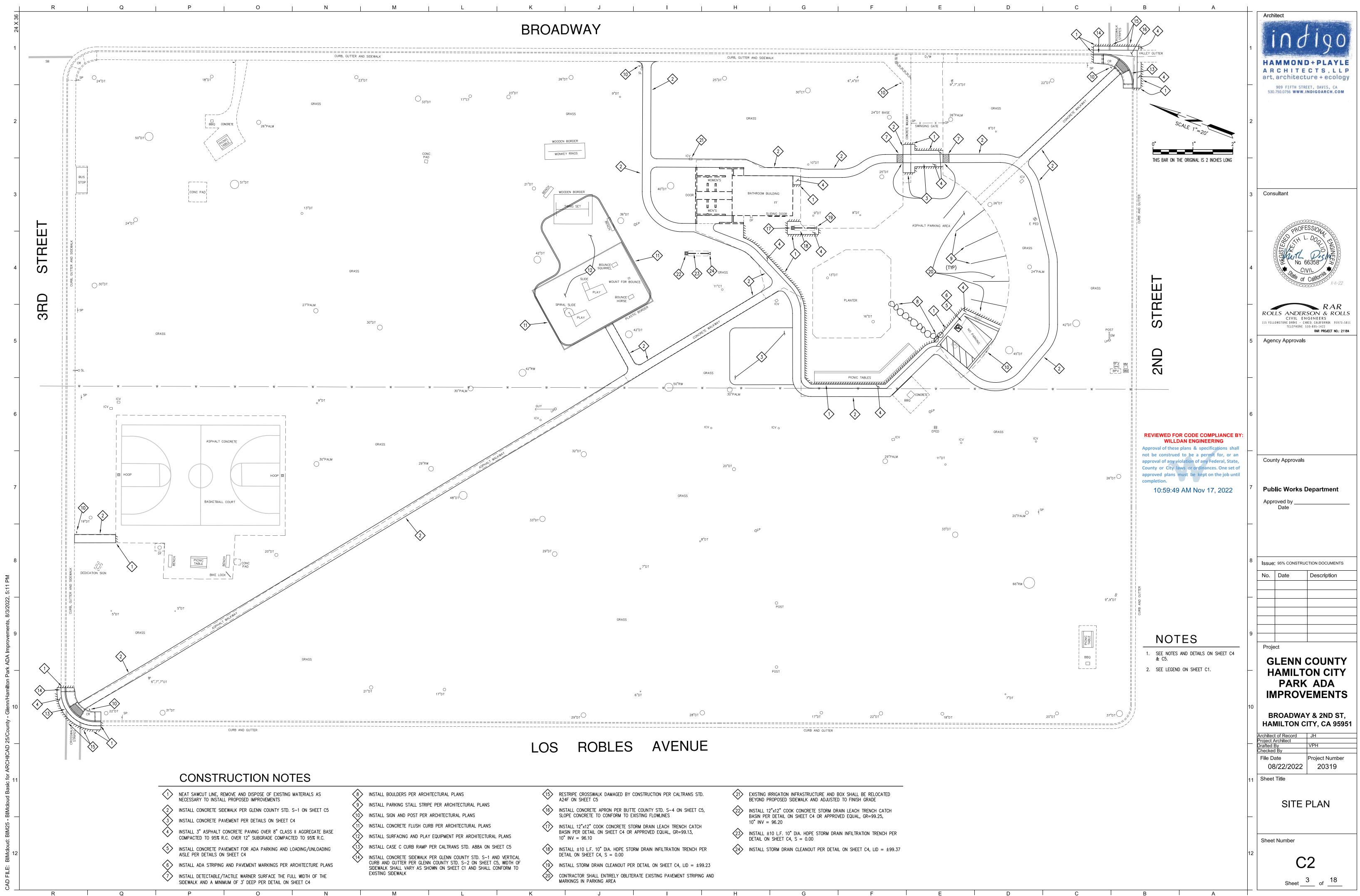
WEIGHT WEST WITH WITHOUT WOOD INSTITUTE WASHINGTON STATE BUILDING CODE

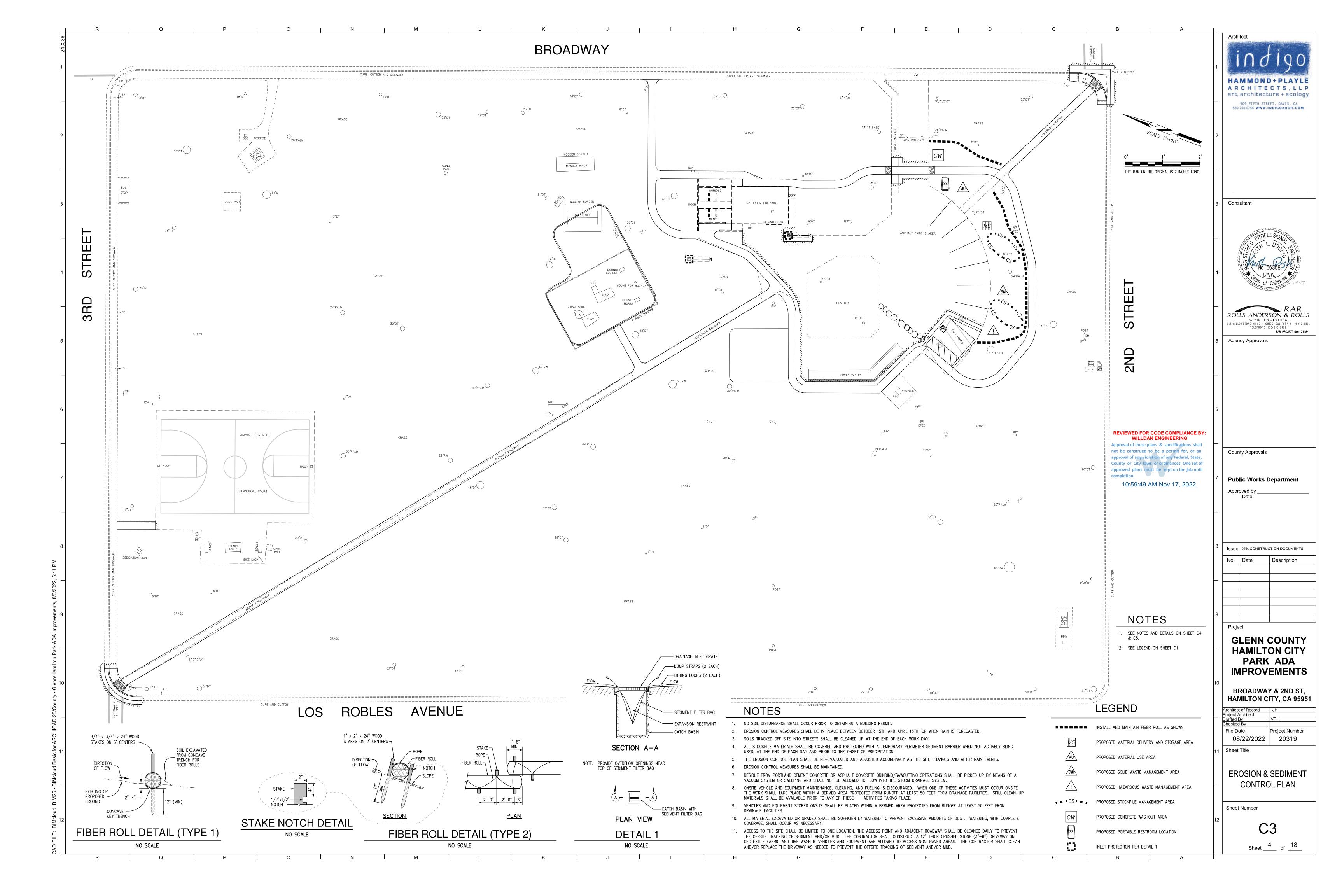


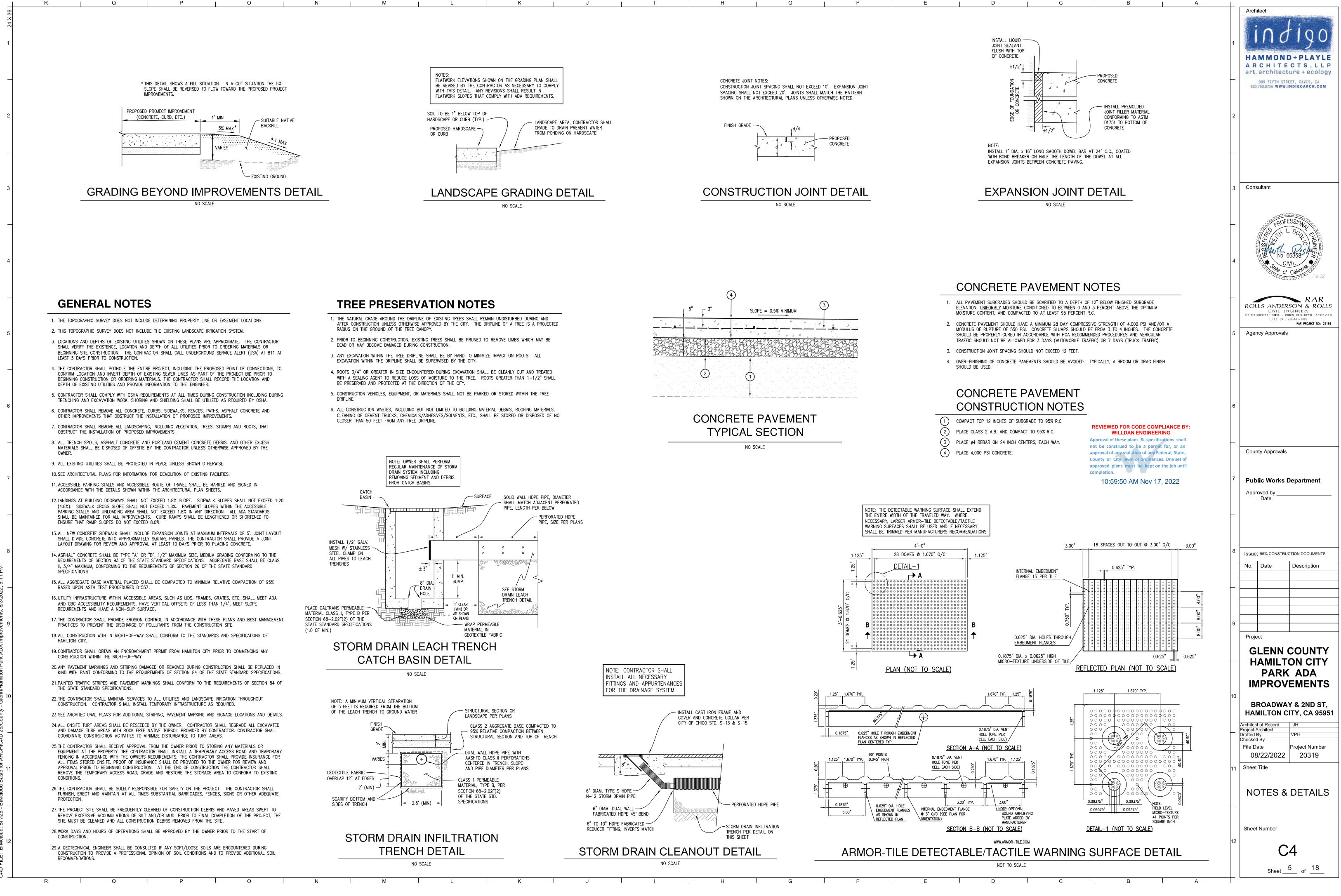
	CONSTRUCTION WILL COMPLY WITH THE FOLLOWING CODES AND ORDINANCES AS AMENDED BY THE STATE OF CALIFORNIA AND LOCAL JURISDICTION.
<u>N POINT</u>	2019 California Administrative Code, CCR, Title 24, Part I 2019 California Building Code, CCR, Title 24, Part 2 (2V) 2019 California Energy Code, CCR, Title 24, Part 6 2019 California Electrical Code (CEC), CCR, Title 24, Part 3 2019 California Mechanical Code (CMC), CCR, Title 24, Part 4 2019 California Plumbing Code (CPC), CCR, Title 24, Part 5 2019 California Fire Code, CCR, Title 24, Part 9 2019 California Green Building Standards Code 2019 California Referenced Standards, CCR, Title 24, Part 12 2019 Title 19 CCR, Public Safety, State Fire Marshal Regulations Glenn County Codes and Regulations
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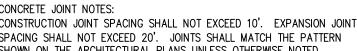
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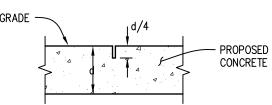


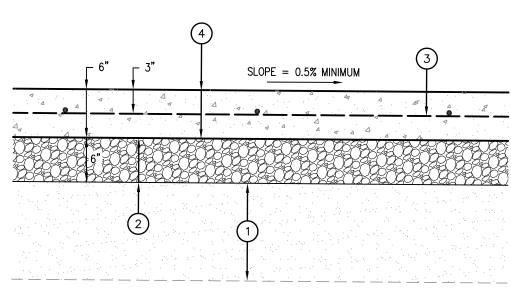




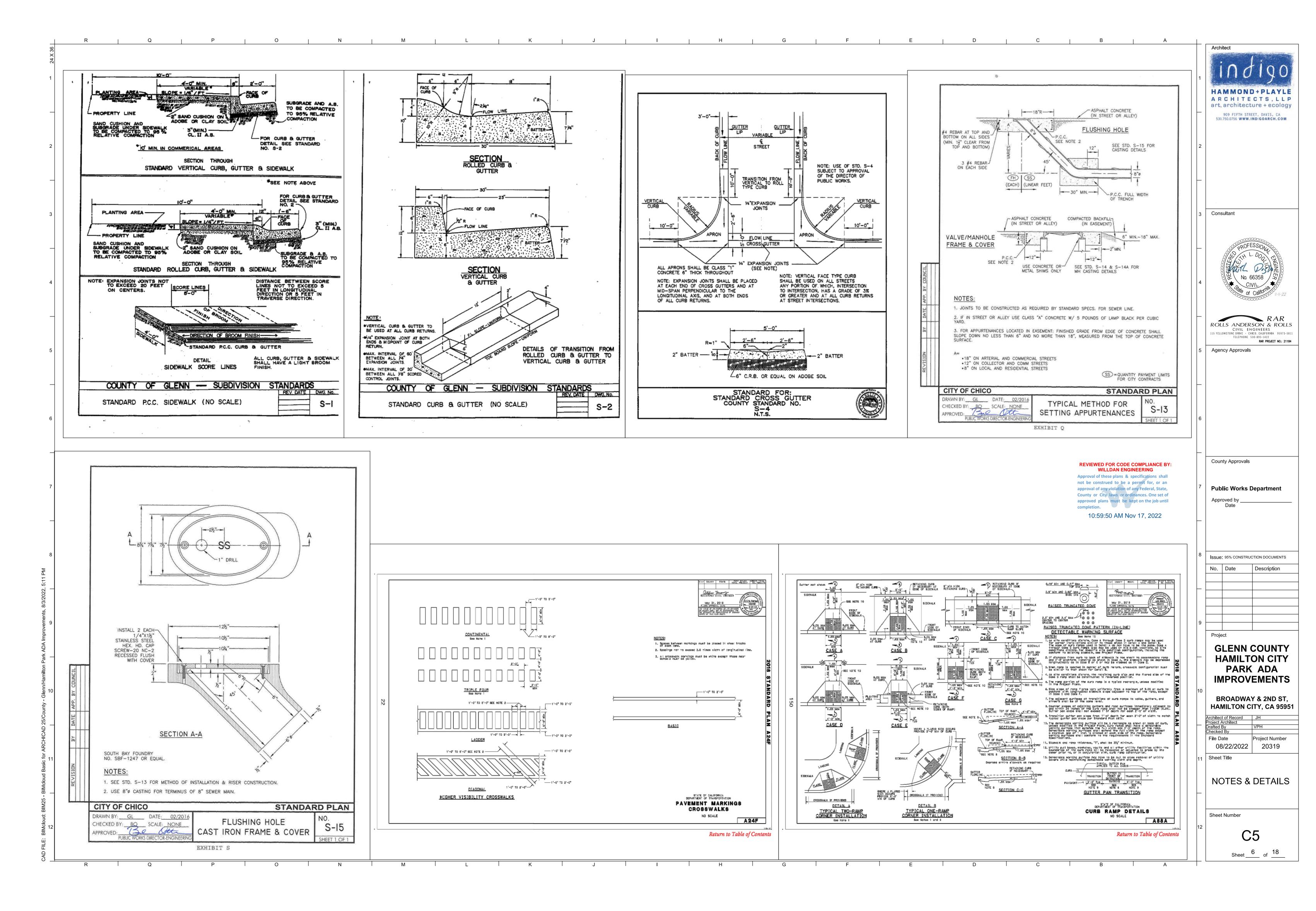












STRUC	TURAL ABBRE		<u>NS</u>
@ AB AC AFF	AT ANCHOR BOLTS ASPHALTIC CONCRETE ABOVE FINISH FLOOR	LFRS LLH LLV LP	LATERAL FORCE RESISTING SYTEM LONG LEG HORIZONTAL LONG LEG VERTICAL LOW POINT
(b) BN BE√ BOC	BELOW BOUNDARY NAILING BEVELED BOTTOM OF	LP LS LT WT LVL	LAG SCREW LIGHT WEIGHT LAMINATED VENEER LUMBER
BOF	CONCRETE BOTTOM OF FOOTING	NIC NTS	MECHANICAL UNIT NOT IN CONTRACT NOT TO SCALE
CIP CJ CJP CL CMU COL	CAST IN PLACE CONSTRUCTION JOINT COMPLETE JOINT PENETRATION CENTER LINE CONCRETE MASONRY UNIT COLUMN	NSG OC OD OSB OWSG OWSJ	NON SHRINK GROUT ON CENTER OUTSIDE DIAMETER ORIENTED STRAND BOARD OPEN WEB STEEL GIRDER OPEN WEB STEEL
CONC CONN CONT	CONCRETE CONNECTION CONTINUOUS	OH PCC	JOIST OPPOSITE HAND PRECAST CONCRETE
	DOUGLAS FIR	PSF	POUNDS PER SQUARE FOOT POUNDS PER
(E) EF EW EJ EOS	EXISTING EACH FACE EACH WAY EXPANSION JOINT EDGE OF SLAB	PT PM	SQUARE INCH PRESSURE TREATED POINT PLYWOOD
EN ES	EDGE NAILING EACH SIDE	R	RADIUS
FA FD FF FLG FN FOC FOM	FRAMING ANCHOR FLOOR DRAIN FINISH FLOOR FLANGE FIELD NAILING FACE OF CONCRETE FACE OF	SAD SDST SIM SCJ SLH	SEE ARCHITECTURAL DRAWINGS SELF DRILLING SELF TAPPING SIMILAR SLIP CONTROL JOINT SHORT LEG HORIZONTAL
FOS	MASONRY FACE OF STUD	SLV SOG	SHORT LEG VERTICAL SLAB ON GRADE
GLB GSM	GLUE LAMINATED BEAM GALVANIZED SHEET	SP SS	STRUCTURAL PLYWOOD STAINLESS STEEL
GT	METAL GIRDER TRUSS	T24	TITLE 24 CALIFORNIA
HAS	HEADED ANCHOR STUD	TOC TOF	CODE TOP OF CONCRETE TOP OF FOOTING TOP OF FRAMING
HDG HP HSB	HOT DIPPED GALVANIZED HIGH POINT HIGH STRENGTH BOLT	TOM T.O. SLAB TOS TOW	TOP OF PRAMING TOP OF MASONRY TOP OF SLAB TOP OF STEEL TOP OF WALL
HSS HT	HOLLOW STRUCTURAL SECTION HIP TRUSS	UNO	UNLESS NOTED OTHERWISE
D	INSIDE DIAMETER	WS WWF	WATER STOP WELDED WIRE
JT	JACK TRUSS	WPJ	FABRIC WEAKENED PLANE JOINT

STRUCTURAL STEEL

(SUBMIT SHOP DRAWINGS BEFORE FABRICATION)

FABRICATION, ERECTION, AND MATERIALS SHALL CONFORM WITH THE AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS AND 2019 CBC. 2. STRUCTURAL STEEL SHAPES SHALL CONFORM TO THE FOLLOWING:

- A. WIDE FLANGE BEAMS & COLUMNS (UNO).....ASTM-A992 (Fy = 50 ksi)
- B. ANGLES (UNO) ASTM-A36
- C. M, S, C, MC, (UNO)ASTM-A36
- D. HP, WT, MT & STASTM-A4922 (Fy = 50 ksi)
- E. RECTANGULAR HSS SHAPES (UNO) ASTM-A500 GRADE C (Fy=50 ksi)
- F. ROUND HSS SHAPES (UNO) ASTM-A500 GRADE C (Fy = 46 ksi)
- G. PIPES (UNO).....ASTM-A53, TYPE E OR S, GRADE B (Fy = 35 ksi)
- H. PLATES, BARS & MISC. (UNO) ASTM-A36 I. ANCHOR RODS (UNO) ASTM-F1554 (Fy=36ksi)
- 3. WELDING DONE BY THE ELECTRIC ARC PROCESS IN ACCORDANCE WITH "AWS". STANDARDS:
- PROVIDE ETOXX ELECTRODES FOR ALL WELDS UNO. USE ONLY CERTIFIED WELDERS. ALL BUTT WELDS SHALL HAVE COMPLETE PENETRATION. ALL EXPOSED BUTT WELDS SHALL BE GROUND. 4. PLACE 2" NON-SHRINK GROUT UNDER ALL BASE PLATES BEFORE ADDING
- VERTICAL LOAD UNO. 5. ALL STRUCTURAL STEEL SHALL BE ERECTED PLUMB AND TRUE TO LINE.
- TEMPORARY BRACING SHALL BE INSTALLED AND SHALL BE LEFT IN PLACE UNTIL OTHER MEANS ARE PROVIDED TO ADEQUATELY BRACE THE STRUCTURE. 6. HOLES FOR UNFINISHED BOLTS SHALL BE OF THE SAME NOMINAL DIAMETER AS
- THE BOLT PLUS 1/16". 7. USE STANDARD AISC GAGE AND PITCH FOR BOLTS UNLESS NOTED OTHERWISE 8. WRAP STRUCTURAL STEEL EMBEDDED IN CONCRETE WITH 6x6-WI.4xWI.4 WWF. DO
- NOT PAINT EMBEDDED AREAS. 9. PROVIDE 3" MINIMUM CONCRETE COVERAGE ON ALL STEEL BELOW GRADE.
- IO. HIGH STRENGTH BOLTS: 3/4" DIAMETER A325-N TYP UNO. II. PROVIDE I/2" STITCH BOLTS & RING FILLS, SPACED AT NOT MORE THAN 2'-O" ON CENTER FOR ALL DOUBLE ANGLE MEMBERS
- 12. COMPOSITE METAL DECKING AND ALL BEAMS AND GIRDERS SHALL REMAIN UNSHORED DURING AND AFTER THE PLACING OF THE CONCRETE FILL UNO.
- 13. DO NOT PAINT THE TOPS OF THE BEAMS & GIRDERS.
- 14. PAINT ALL EXPOSED STEEL W/ PRIMER.

FORCE 5 SYTEM AL EM IGHT UMBER

SLAB ON GRADE TIE BEAMS AND FOUNDATIONS CONCRETE AGGREGATES:

FOLLOWS:

- PROHIBITED. SEE REBAR WELDING NOTE.
- 8. WIRE FABRIC SHALL CONFORM TO ASTM AIO64-18.
- FOLLOWS, UNO ON DRAWINGS: SLABS (ON GROUND)
- THESE DRAWINGS. II. GENERAL:
- A. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE SLABS OR WALLS UNLESS SPECIFICALLY DETAILED.
- AND GROUNDS TO BE CAST IN CONCRETE.
- POUR WITH A FINE SPRAY.
- COVERED W/ CONC.
- PLACEMENT.
- DIRECTION.

ROOF TRUSS NOTES-ALTERNATE BID

- TPI-I-2014 OF THE TRUSS PLATE INSTITUTE, UNO.

- SIZE, TYPICAL ALL TRUSSES UNO.
- REQUIREMENTS.

LOADING:

20 PSF LIVE LOAD - TOP CHORD - REDUCIBLE 13.5 PSF DEAD LOAD - TOP CHORD

6 PSF DEAD LOAD - BOTTOM CHORD (ALL TRUSSES) IO PSF LIVE LOAD - BOTTOM CHORD NON-CONCURRENT WITH TOP CHORD LIVE LOAD (ALL TRUSSES)

TRUSS FABRICATOR SHALL INCORPORATE ONE 400# (250#LL+150#DL) VERTICAL LOAD ON THE BOTTOM CHORD OF ALL TRUSSES FROM FIRE SPRINKLER LINE. TRUSS MUST BE DESIGNED TO CARRY THE LOAD AT ANY POINT ALONG THE BOTTOM CHORD. THE 250#LL IS NON- CONCURRENT WITH THE ROOF LIVE LOAD.

WIND TO BE CALCULATED PER CRITERIA SHOWN ON STRUCTURAL DRAWINGS.

GABLE-END WALL TRUSSES SHALL BE DESIGNED TO TRANSFER 275 PLF HORIZONTAL LOAD BETWEEN TOP AND BOTTOM CHORD (WIND/SEISMIC SHEAR TRANSFER), UNO.

FABRICATOR TO DESIGN ALL DT (DRAG TRUSSES) TO TRANSFER INDICATED HORIZONTAL LOAD BETWEEN TOP & BOTTOM CHORDS, TYP.

- CONNECTIONS.
- A) IDENTITY OF THE COMPANY MANUFACTURING THE TRUSS B) THE DESIGN LOAD
- C) THE SPACING OF THE TRUSSES
- REQUIRED.
- SPECIFIED BY TRUSS FABRICATOR.
- APPROVED BY GENERAL CONTRACTOR. LARGER
- AND UTILITY CLEARANCES REQUIRED.

CONCRETE AND REINFORCING STEEL:

(SUBMIT REBAR SHOP DRAWINGS PRIOR TO FABRICATION) CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 318-14 AS MODIFIED BY CBC. 2. THE MINIMUM 28 DAY STRENGTH AND TYPE OF CONCRETE SHALL BE AS

> 4000 PSI (150 PCF) 3000 PSI (150 PCF)

CEMENT SHALL CONFORM TO ASTM CI50-20, TYPE II - V.

NATURAL SAND AND ROCK AGGREGATES SHALL CONFORM TO ASTM C33-18. 5. REINFORCING SHALL CONFORM TO ASTM A615 -- GRADE 60. UNO 6. WELDING OF REINFORCING STEEL SHALL CONFORM TO AWS DI.4-18 USING PROPER LOW HYDROGEN ELECTRODES. TACK WELDING TO REBAR IS STRICTLY

REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND INSTALLED ACCORDING TO "MANUAL OF STANDARD PRACTICE OF REINFORCED CONCRETE CONSTRUCTION" BY THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI)

9. DIMENSIONS SHOWN FOR LOCATION OF REINFORCING ARE TO THE FACE OF MAIN BARS AND DENOTE CLEAR COVERAGE. CONCRETE COVERAGE SHALL BE AS

CONCRETE DEPOSITED DIRECTLY AGAINST GROUND (EXCEPT SLABS) 3" CONCRETE EXPOSED TO GROUND BUT PLACED IN FORMS 2" .. POSITION IN CENTER OF SLAB

IO. ALL BARS SHALL HAVE A CLASS B MINIMUM SPLICE LAP UNO. SEE TABLE IN

B. REFER TO ARCHITECTURAL, STRUCTURAL, CIVIL, ELECTRICAL AND MECHANICAL DRAWINGS FOR ALL MOULDS, GROOVES, ORNAMENTS, CLIPS

12. CONSTRUCTION JOINTS SHALL BE MADE ROUGH AND ALL LAITANCE REMOVED FROM THE SURFACE. CONCRETE MAY BE ROUGHENED BY CHIPPING THE ENTIRE SURFACE, SANDBLASTING OR HOSING THE SURFACE 4 TO 6 HOURS AFTER THE

13. REMOVE ALL DEBRIS FROM THE FORMS BEFORE PLACING ANY CONCRETE. 14. REINFORCING, DOWELS, BOLTS, ANCHORS, SLEEVES, ETC. TO BE EMBEDDED IN CONCRETE SHALL BE SECURELY POSITIONED BEFORE PLACING CONCRETE. OBTAIN APPROVAL OF ALL AFFECTED TRADES PRIOR TO PLACING CONCRETE. 15. MAXIMUM FREE FALL OF CONCRETE SHALL BE 4'-O".

16. WALLS SHALL BE PLACED IN HORIZONTAL LAYERS OF 2'-O" MAX DEPTH 17. NO WOOD SPREADERS ALLOWED. NO WOOD STAKES ALLOWED IN AREAS TO BE

18. CONCRETE MIX DESIGN SHALL BE PREPARED PER CBC CHAPTER 19 AND REVIEWED BY THE STRUCTURAL ENGINEER AT LEAST 3 WORKING DAYS PRIOR TO

19. WELDED WIRE FABRIC SHALL BE LAP SPLICED TWO SQUARES MIN. EACH

20. NOTIFY THE STRUCTURAL ENGINEER 48 HOURS PRIOR TO PLACING CONCRETE. 21. CONTRACTOR TO SUBMIT PROPOSED CONTROL AND CONSTRUCTION JT LOCATION TO STRUCTURAL ENGINEER PRIOR TO CONCRETE POUR. SPACING SHALL BE BETWEEN 24 AND 30 TIMES THE SLAB THICKNESS MAXIMUM.

TRUSSES ARE PREFABRICATED GANG-NAIL PLATE CONNECTED 2x MEMBER ASSEMBLY 24" OC TYPICAL SPACING UNO. DESIGN AND FABRICATION SHALL CONFORM TO CBC. PLATES SHALL HAVE AN ICC LISTING AND CONFORM TO

THE FABRICATOR SHALL PROVIDE SHOP DRAWINGS/CALCULATIONS AND LAYOUT PLAN (STAMPED AND SIGNED BY A CA REGISTERED PROFESSIONAL ENGINEER) TO THE ARCHITECT AND BUILDING DEPARTMENT. SHOP DRAWINGS AND

CALCULATIONS SHALL BE APPROVED PRIOR TO FABRICATION. TRUSS BEARING WALLS ARE NOTED ON THE ROOF FRAMING PLANS; TYPICALLY,

TRUSSES SPAN FROM EXTERIOR WALL TO EXTERIOR WALL. 4. I.25 DURATION OF LOAD FACTOR IS PERMITTED WHERE APPLICABLE. NO REPETITIVE MEMBER STRESS INCREASE IS ALLOWED.

TOP AND BOTTOM CHORD MEMBER SHALL BE 2×6 DF #2 MINIMUM GRADE AND 6. TRUSS FABRICATOR SHALL CLEARLY SPECIFY AND DETAIL ALL TRUSS BRACING

ROOF TOP MECH UNIT WEIGHTS HAVE NOT BEEN INCLUDED IN LOADS. TRUSS FABRICATOR TO REFER TO MECH DWGS & INCLUDE FOR DESIGN

DEAD LOADS DO NOT INCLUDE TRUSS SELF WT.

8. TRUSSES SHALL HAVE SIMPSON HI CLIP AT ALL EXTERIOR BEARING

9. EACH PREFABRICATED TRUSS SHALL BE LEGIBLY BRANDED, MARKED OR OTHERWISE HAVE PERMANENTLY AFFIXED THERETO THE FOLLOWING INFORMATION LOCATED ON THE FACE OF THE BOTTOM CHORD:

EFFECTS OF ECCENTRIC LOADING SHALL BE CONSIDERED IN THE DESIGN OF ALL JOINTS. ALTERNATIVELY, INCREASE THE CONNECTOR PLATE SIZE BY 25%. 12. GENERAL CONTRACTOR TO PROVIDE WEB BRACING AS REQUIRED BY TRUSS FABRICATOR DESIGN AND PROVIDE TEMPORARY ERECTION BRACING AS

13. BUILT-UP GIRDER TRUSSES SHALL BE LAMINATED TOGETHER TO TRANSFER ALL LOADS AND TO ENSURE ADEQUATE LOAD SHARING. CONNECTION BETWEEN TRUSSES TO BE DESIGNED AND SPECIFIED BY THE TRUSS DESIGNER. 14. ALL HARDWARE REQUIRED FOR CONNECTING TRUSSES (JACK TO HIP, HIP TO GIRDER, GIRDER TO GIRDER, ETC.) SHALL BE DESIGNED, DETAILED AND

15. GENERAL CONTRACTOR TO VERIFY ALL DIMENSIONS SHOWN ON TRUSS PROFILES WITH ARCHITECTURAL DRAWINGS AND IN FIELD WITH WALL LAYOUT PRIOR WITH ARCHITECTURAL DRAWINGS AND IN FIELD WITH WALL LAYOUT PRIOR TO FABRICATION. PROVIDE SHOP DRAWINGS WITH DIMENSIONS REVIEWED AND

16. TRUSS PROFILES TO INCLUDE OVERHANGS AS REQ'D. COORDINATE W/ PLANS. 17. WOOD UNDER PLATES MUST BE FREE OF KNOT HOLES AND KNOTS 3/8" AND

18. TRUSS WEB CONFIGURATIONS SHOWN ON STRUCTURAL OR ARCHITECTURAL DRAWINGS HEREIN ARE SYMBOLIC ONLY, UNO. WEB CONFIGURATION SHALL BE DESIGNED BY THE TRUSS FABRICATOR IN ACCORDANCE WITH APPLICABLE CODES AND PROJECT REQUIREMENTS. GENERAL CONTRACTOR SHALL COORDINATE AMONG TRADES AND TRUSS FABRICATOR REGARDING ACCESS

WOOD:

(SUBMIT SHOP DRAWINGS BEFORE FABRICATION OF GLU-LAM MEMBERS) ALL STRUCTURAL WOOD SHALL CONFORM WITH THE FOLLOWING SPECIFICATIONS: DOUGLAS FIR - LARCH WESTERN LUMBER GRADING RULES WWPA. GLUED LAMINATED BEAMS ANSI A190.1 ANSI 405 ANSI 117 U.S. PRODUCT STANDARD PS 1-09 FOR

SOFT PLYWOOD.

CANTILEVERS & CONTINUOUS CONDITIONS.

STRUCT | SHEATHING, 5 PLY 32/16, EXPOSURE |

ALL ROOF BEAMS SHALL HAVE

WALL PLYWOOD: 15/32" APA RATED

3000 FT RADIUS CAMBER UNO.

DF#I TYPICAL

PLYWOOD

2. MINIMUM GRADES SHALL BE: STRUCTURAL FRAMING

MOISTURE CONTENT TO BE < 19% AT TIME OF CONSTRUCTION GLUED LAMINATED MEMBERS COMBINATION 24F-V4 FOR SIMPLE SPANS AND COMBINATION 24F-V8 FOR

STRUCTURAL PLYWOOD (UNO)

- ROOF PLYWOOD: 15/32" APA RATED STRUCT | SHEATHING, 5 PLY, 32/16, EXPOSURE 1 3. WALLS SHALL HAVE DOUBLE TOP PLATES, LAPPED AT WALL & PARTITION INTERSECTION WITH 3-16d NAILS. SPLICE UPPER AND LOWER PLATES WITH 'MIN' SPLICE AS SHOWN IN TYPICAL DETAIL, UNO.
- PROVIDE SOLID BLKG BETWEEN JOISTS OR RAFTERS AT ALL SUPPORTS. CUTTING OF WOOD JOISTS SHALL BE LIMITED TO CUTS AND BORED HOLES NOT DEEPER THAN ONE-FIFTH THE JOIST DEPTH FROM THE TOP, \sharp Located
- NOT FARTHER FROM THE END THAN THREE TIMES THE JOIST DEPTH. HOLES FOR BOLTS IN WOOD SHALL BE BORED WITH A BIT OF THE SAME
- NOMINAL DIAMETER AS THE BOLT + 1/16".
- HOLES FOR LAG SCREWS SHALL BE FIRST BORED TO THE SAME NOMINAL DIAMETER & DEPTH AS THE SHANK. THE REMAINDER OF THE HOLE SHALL BE NO LARGER THAN THE ROOT OF THE THREAD.
- 8. LAG SCREWS AND WOOD SCREWS SHALL BE SCREWED AND NOT DRIVEN INTO PLACE. 9. ALL BOLTS AND LAG SCREWS SHALL BE PROVIDED WITH METAL WASHERS
- UNDER HEADS & NUTS WHICH BEAR ON WOOD. APPLIES ALSO TO INSERTED EXPANDING FASTENERS - KWIK-BOLT, STRONG BOLT, ETC.

BOLT-DIA	ROUND WASHER	SQUARE WASHER
I/2"	3" DIA x 3/16"	3" SQ × .195"
5/8"	3" DIA x I/4"	3" SQ × .25"
3/4"	3" DIA x I/4"	3" SQ x .315"
7/8"	3 /2" DIA x 5/16"	3" 5Q × .315"
"	4" DIA x 3/8"	3 1/2" SQ x .39"
7/8"	3 1/2" DIA x 5/16"	3" SQ x .315"

- IO. ALL BOLT & LAG SCREWS SHALL BE TIGHTENED AT TIME OF INSTALLATION AND RE-TIGHTENED BEFORE CLOSING IN OR AT COMPLETION OF JOB. LAY ALL STRUCTURAL PLYWOOD ON ROOF AND FLOORS WITH FACE GRAIN
- PERPENDICULAR TO SUPPORTS. 12. BLOCK SP JOINTS WITH 3 X 4 FLAT BLOCKING WHERE NOTED ON FRAMING PLANS AND WITH BLOCKING SAME SIZE AS STUDS AT WALLS.
- 13. CROSS BRIDGING OR FULL DEPTH BLOCKING BETWEEN JOISTS OR RAFTERS 2XIO & LARGER REQUIRED AT 8'-O" O.C. MAXIMUM.
- 14. WHERE FRAMING HANGERS ARE REQUIRED & ARE NOT SHOWN ON SECTIONS, DETAILS OR PLANS THE FOLLOWING SIMPSON HANGERS SHALL BE USED. SLOPE, SKEW, TURN IN FLANGES & PROVIDE TOP FLANGE HANGERS AS REQD.

	2X & 3X MEMBERS	U HANGERS
	4X MEMBERS	HU HANGERS
	6X MEMBERS	HUTF HANGERS
	I JOIST MEMBERS	BA HANGERS
	GLU LAM MEMBERS	LEG HANGERS
	4x \$ 6x POSTS	PCZ/EPCZ POST CAPS
15	5. ALL METAL HARDWARE SHALL B	E MANUFACTURED BY SIMPSON STRONG TIE

COMPANY. ALL ITEMS SHALL BE INSTALLED PER SIMPSON SPECIFICATIONS. FILL ALL HOLES OF METAL HARDWARE WITH SPECIFIED FASTENERS, UNO. 16. WOOD SYMBOLS:

CONTINUOUS DECKING

17. NAILS FOR ALL STRUCTURAL FRAMING SHALL BE AS SPECIFIED BELOW. STRUCTURAL NAILS

MARK	NAIL TYPE	DIA.	LENGTH	
8d	8d COMM	0.131"	2 /2"	
lOd	IOd COMM	0.148"	З"	
16d	16d COMM	0.162"	3 1/2"	
20d	20d COMM	0.192"	4"	

18. ALL FASTENERS FOR PRESSURE-PRESERVATIVE TREATED & FIRE-RETARDANT

- TREATED WOOD SHALL BE HOT-DIPPED GALVANIZED OR STAINLESS STEEL. 19. SILL BOLTS TO HAVE SQUARE STEEL WASHERS AS INDICATED IN TABLE ABOVE
- 20. ALL WOOD MEMBERS IN DIRECT CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED. MATERIAL TREATED W/ ARSENIC CONTENT ARE NOT NOT PERMITTED (CCA & ACA) 21 MINIMUM FASTENING OF SHEATHING TO SUPPORTING MEMBERS SHALL BE

		EDCE			
	AS FOLLOWS UNI	ESS NOTED OTHER	RWISE ON DRAWING	GS.	
∠ı.	MINIMUM FASTEN	ING OF SHEATHING	TO SUPPORTING I	Members Shall e	シニ

SHEATHING THICKNESS 't'	EDGE FASTENING	FIELD FASTENING	
't' <u><</u> 3/8"	8d @ 6" O.C.	8d @ 12" O.C.	WOOD
3/8" < 't' < 3/4"	10d @ 6" 0.C.	10d @ 12" O.C.	NOOD
't' <u><</u> 3/8"	#8 FLATHEAD SDS @ 6" O.C.	#8 FLATHEAD SDS @ 12" O.C.	COLD FORMED
3/8" < 't' < 3/4"	#8 FLATHEAD SDS @ 6" O.C.	#8 FLATHEAD SDS @ 12" <i>O</i> .C.	STEEL

FOUNDATIONS: (TABLE 1806.2)

- . IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO SHORE AND BRACE AS REQUIRED.
- 2. CONTRACTOR SHALL PROVIDE FOUNDATIONS CONSTRUCTED WITHIN A LEVEL ALIGNMENT TOLERANCE OF +/- 1/2" IN 20'-0" FOR SUPPORT OF
- MASONRY 3. ALL FOUNDATIONS ARE SHOWN SHOWN AND DIMENSIONED AS BEING FORMED. FOUNDATIONS MAY BE PLACED IN NEAT EXCAVATIONS
- PROVIDED FOOTINGS ARE INCREASED 2" IN WIDTH. 4. EXCAVATIONS SHALL BE CLEANED OF ALL DEBRIS. STANDING WATER SHALL BE REMOVED.
- 5. ALLOWABLE SOIL BEARING PRESSURE (D & L) = 1500 pst, BASED ON TABLE 1806.2 OF CBC
- 6. ALL COMPACTED FILL SHALL HAVE A MINIMUM RELATIVE COMPACTION OF 90 PERCENT.
- 7. NOTIFY THE STRUCTURAL ENGINEER 48 HOURS IN ADVANCE OF PLACING CONCRETE.

TYPICAL NOTES APPLICABLE TO ALL DRAWINGS UNLESS. NOTED OR SHOWN OTHERWISE

GENERAL NOTES:

- CONSTRUCTION SHALL CONFORM TO THE 2019 CALIFORNIA BUILDING CODE, CBC. 2. NOTES AND DETAILS ON TYPICAL SHEETS SHALL APPLY UNLESS OTHERWISE SHOWN
- OR NOTED ON PLANS. 3. CONTRACTOR SHALL NOT SCALE DRAWINGS FOR SIZES, LENGTHS, CLEARANCES, ETC.
- 4. DETAILS OF CONSTRUCTION NOT FULLY SHOWN SHALL BE OF THE SAME NATURE AS SHOWN FOR A SIMILAR CONDITION 5. PRIOR TO FABRICATION, SHOP DRAWINGS, SHALL BE SUBMITTED FOR REVIEW BY THE
- STRUCTURAL ENGINEER ON ALL STRUCTURAL STEEL, REINFORCING STEEL, STAIRS, GLUE-LAMINATED BEAMS, CONCRETE MIX PROPORTIONS. SHOP DRAWINGS: SHOP DRAWING SUBMITTALS PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS AND THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT BY INDICATING WHICH MATERIAL HE INTENDS TO FURNISH AND INSTALL AND BY DETAILING THE FABRICATION AND INSTALLATION METHODS INTENDED FOR USE. DUPLICATION OF DESIGN DRAWINGS FOR THE PURPOSE OF SHOP DRAWINGS IS NOT ACCEPTABLE. 6. SAFETY NOTE:
- A. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COMPLY WITH THE PERTINENT SECTIONS OF THE "CONSTRUCTION SAFETY ORDERS" ISSUED BY THE STATE OF CALIFORNIA, LATEST EDITION, AND ALL OSHA REQUIREMENTS AS THEY APPLY TO THIS PROJECT.
- B. THE STRUCTURAL ENGINEER DOES NOT ACCEPT ANY RESPONSIBILITY FOR THE CONTRACTOR'S FAILURE TO COMPLY WITH THESE REQUIREMENTS. C. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADEQUATE DESIGN AND
- CONSTRUCTION OF ALL FORMS AND SHORING REQUIRED.
- 7. CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, PROPERTY LINES, ETC.
- ON THE JOB. 8. CONTRACTOR SHALL NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER WHERE A CONFLICT OCCURS ON ANY OF THE CONTRACT DRAWINGS OR DOCUMENTS. CONTRACTOR IS NOT TO ORDER MATERIAL OR CONSTRUCT ANY PORTION OF THE BUILDING THAT IS IN CONFLICT UNTIL SAID CONFLICT IS RESOLVED WITH THE AFFECTED PARTIES. IF NOT RESOLVED PRIOR TO BID, THE MOST STRINGENT CONDITION WILL APPLY.
- 9. REVIEW OF FIRE SPRINKLER SHOP DRAWINGS, CALCULATIONS AND THE FOLLOW-UP CERTIFICATION LETTER REQUIRED BY THE FIRE MARSHALL IS NOT INCLUDED IN THE SERVICES OF THE STRUCTURAL ENGINEER OF RECORD. THE COST OF THIS REVIEW WILL BE CHARGED TO THE SUBCONTRACTOR RESPONSIBLE FOR THE DESIGN. THIS FEE MUST BE RECEIVED BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO THE COMPLETION OF THIS TASK.

REVIEWED FOR CODE COMPLIANCE BY:

WILLDAN ENGINEERING

Approval of these plans & specifications shall

not be construed to be a permit for, or an

approval of any violation of any Federal, State,

County or City laws or ordinances. One set of

approved plans must be kept on the job until

10:59:50 AM Nov 17, 2022

completion.

DESIGN LOADS:

CODE: 2019 CALIFORNIA BUILDING CODE (CBC)

LIVE LOADS:

ROOF 20.0 PSF (REDUCIBLE) SOLAR LOADS:

ROOF SOLAR 3.5 PSF

WIND:

BASIC WIND SPEED ∨ (3 SEC GUST)= <u>94 MPH</u> RISK CATEGORY:

EXPOSURE <u>C</u>.

ENCLOSURE CLASSIFICATION:	INTERNAL PRESSURE COEFFICIENT (GCpi)
	+0.18, -0.18
PARTIALLY ENCLOSED	+0.55, -0.55
PARTIALLY OPEN	+0.18, -0.18
OPEN	0.00

VELOCITY PRESSURE 9h = 16.3 PSF

COMPONENTS & CLADDING:

*WIND PRESSURE FOR BUILDING ELEMENTS (16.0 PSF MINIMUM)

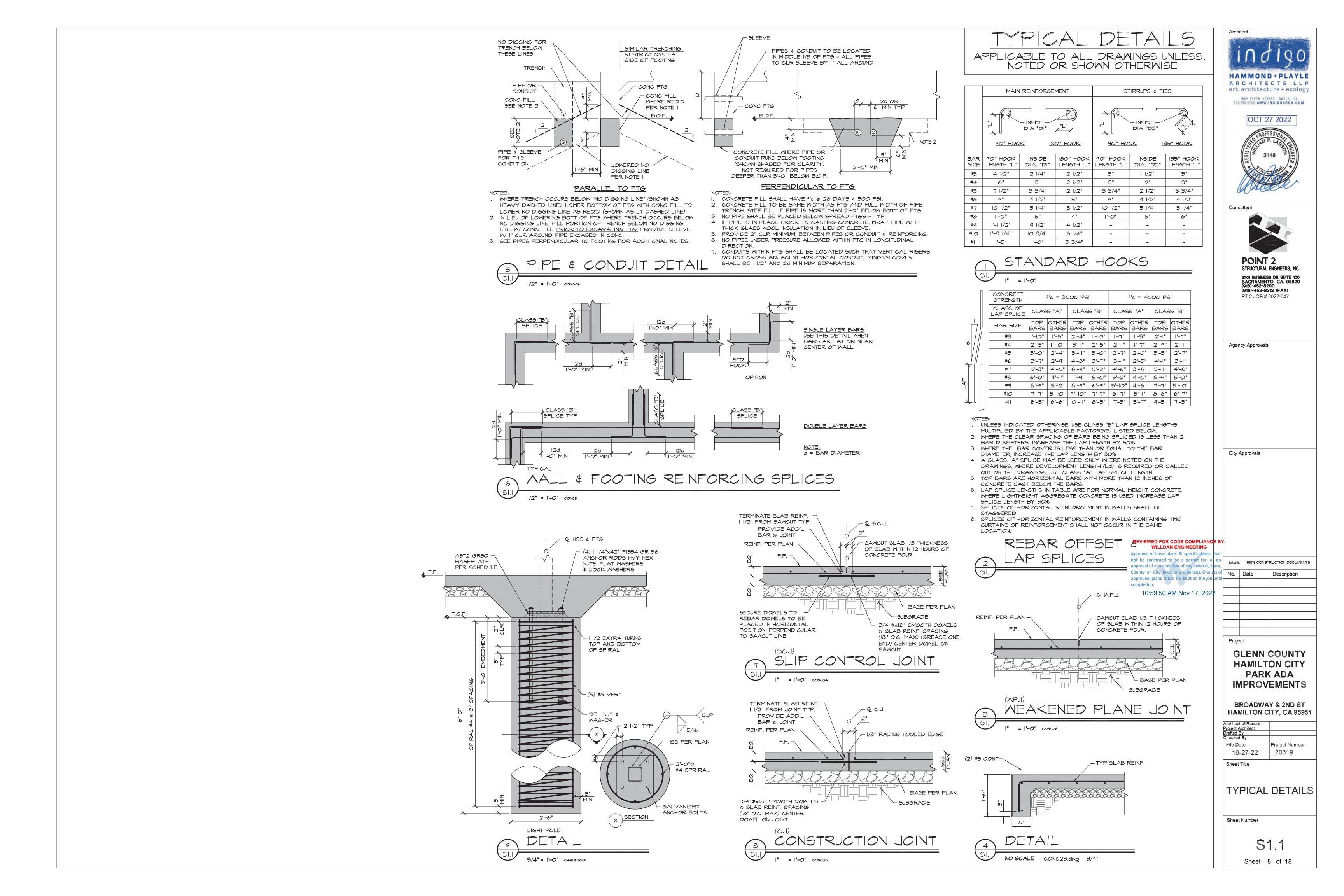
DESIGN WIND	PRESSURE (PSF)	* DESIGN PRESSURE IS FOR EFFECTIVE WIND AREA < 10 SQ FT. PRESSURE CAN			
<u>R00F</u>	-	BE REDUCED FOR LARGER AREAS AS PER ASCE 7-16			
ZONE I	I6.I, - I6	** - PRESSURE FOR < 2.0 SQ FT EFF AREA			
ZONE 2	24.8, -22.3				
ZONE 3	32.2, -28.9				

SEISMIC

	BASIC SEISMIC RESISTING TYPE: <u>62 .</u> DESCRIPTION: <u>STEEL ORI</u>		NTILEVER COLUI	LA	DING LOC, TITUDE: <u>39</u> NGITUDE: -	
Ħ I	BEISMIC IMPORTANCE =ACTOR I _E 1.00 1.25 1.50	SITE CLASS A B C C D E F	RISK CATEGORY 			
C	MAPPED MAXIMUM CONSIDERED SPECTRAL RESPONSE ACCELERATION	NS: S ₅ = 0. S ₁ = 0.3				
ħ	DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS:	NS S _{DS} = (S _{DI} = (
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Sheet 7 of 18



FABLE NO. 2304.10.1 F		
. BLKG. BTWN CLG JOISTS, TRUSSES OR RAFTERS TO TOP PLATE		TOENAIL EA END
2. CEILING JOISTS TO TOP PLATE	3-8d COMMON (2 1/2"x0.131")	TOENAIL, EA JOIST
3. CEILING JOISTS, LAPS OVER PARTITIONS (SEE SEC. 2308.7.3.1, TABLE 2308.7.3.1)	3-16d COMMON (3 1/2"x0.162")	FACE NAIL
4. CEILING JOISTS TO PARALLEL RAFTERS (SEE SEC. 2308.7.3.1, TABLE 2308.7.3.1)	SEE TABLE 2308.7.3.1	FACE NAIL
5. COLLAR TIE TO RAFTER	3-10d COMMON (3"x0.148")	FACE NAIL
6. RAFTER OR ROOF TRUSS OR PLATE (SEE SEC. 2308.7.5, TABLE 2308.7.5)	3-10d COMMON (3"x0.148")	TOENAIL
7. JACK RAFTER TO HIP OR VALLEY OR ROOF RAFTER TO 2x RIDGE BEAM	3-10d COMMON (3"x0.148") 2-16d COMMON (3 1/2"x0.162")	TOENAIL END NAIL
8. STUD TO STUD	16d COMMON (3 1/2"x0.162") @ 24" O.C.	FACE NAIL
9. BUILT-UP CORNER STUDS	16d COMMON (3 1/2"x0.162")	16" O.C. FACE NAIL
I. CONTINUOUS HEADER TO STUD	4-8d COMMON (2 1/2"x0.131")	TOENAIL
2. TOP PLATE TO TOP PLATE	6d COMMON (3 /2"×0. 62") @ 6" 0.C.	TYP. FACE NAIL
4. BOTT PL TO JOIST, RIM JOIST, BAND JOIST OR BLKG.	16d COMMON (3 1/2"x0.162") @ 16" O.C.	TYP. FACE NAIL
5. BOTT PL TO JOIST, RIM JOIST, BAND JOIST OR BLKG.	2-16d COMMON (3 1/2"x0.162") @ 16" O.C.	TYP. FACE NAIL
6. STUD TO TOP OR BOTTOM PLATE	4-8d COMMON (2 1/2"x0.131") 2-16d COMMON (3 1/2"x0.162")	TOENAIL END NAIL
7. TOP OR BOTTOM PLATE TO STUD	2-16d COMMON (3 1/2"x0.162")	END NAIL
8. TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	2-16d COMMON (3 1/2"x0.162")	FACE NAIL
9. I" DIAG. BRACE TO EA. STUD AND PLATE	2-8d COMMON (2 1/2"x0.131")	FACE NAIL
21. 1"x8" AND WIDER SHEATHING TO EA. BEARING	3-8d COMMON (2 1/2"x0.131")	FACE NAIL
22. JOIST TO SILL, TOP PLATE OR GIRDER	3-8d COMMON (2 1/2"x0.131")	TOENAIL
23. RIM JOISTBAND JOIST, OR BLOCKING TO TOP PLATE, SILL OR FRMG BELOW	8d COMMON (2 1/2"x0.131") @ 6" 0.C.	TOENAIL
24. I"x6" SUBFLOOR OR LESS TO EA. JOIST	2-8d COMMON (2 1/2"x0.131")	FACE NAIL
25. 2" SUBFLOOR TO JOIST OR GIRDER	2-16d COMMON (3 1/2"x0.162")	FACE NAIL
26. 2" PLANKS	16d COMMON (3 1/2"×0.162")	@ EA. BEARING FACE NAIL
27. BUILT-UP GIRDER AND BEAMS (2" LUMBER LAYERS)	20d COMMON (4"x0.192") @ 32" O.C.	FACE NAIL @ TOP & BO STAGG. ON OPP. SIDES FACE NAIL @ ENDS & @
	2-20d COMMON (4"x0.192")	EA. SPLICE
28. LEDGER STRIP SUPPORTING JOIST OR RAFTERS	3-16d COMMON (3 1/2"×0.162")	FACE NAIL @ EA. JOIST OR RAFTER
29. JOIST TO BAND JOIST OR RIM JOIST	3-16d COMMON (3 1/2"x0.162")	END NAIL
30. BRIDGING OR BLOCKING TO JOIST OR STUD	2-8d COMMON (2 1/2"×0.131")	TOENAIL EA. END
31. WOOD STRUCTURAL PANELS SUBFLOOR, ROOF AND WALL SHEATHING TO FRAMING AND PARTICLEBOARD WALL SHEATHING TO FRAMING ^C	1/2" AND LESS 6d COMMON 19/32" TO 3/4" 8d COMMON 7/8" TO 1 1/4" 10d COMMON	6" @ EDGES 12" @ INTERMEDIATE SUPPORTS
SINGLE FLOOR (COMBINATION SUBFLOOR- UNDERLAYMENT TO FRAMING)	3/4" OR LESS 8d COMMON 7/8" TO I" 8d COMMON I I/8" TO I I/4" IOd COMMON	
34. FIBERBOARD SHEATHING ^{CD}	1/2" THICK I 1/2" GALV ROOFING NAIL 25/32" THICK I 3/4" GALV ROOFING NAIL	
39. PANEL SIDING (TO FRAMING) ^C	1/2" OR LESS 6d COMMON 5/8" 8d COMMON	
42. INTERIOR PANELING	1/4" 4d CASING (1 1/2"×0.080") 3/8" 6d CASING (2"×0.099")	

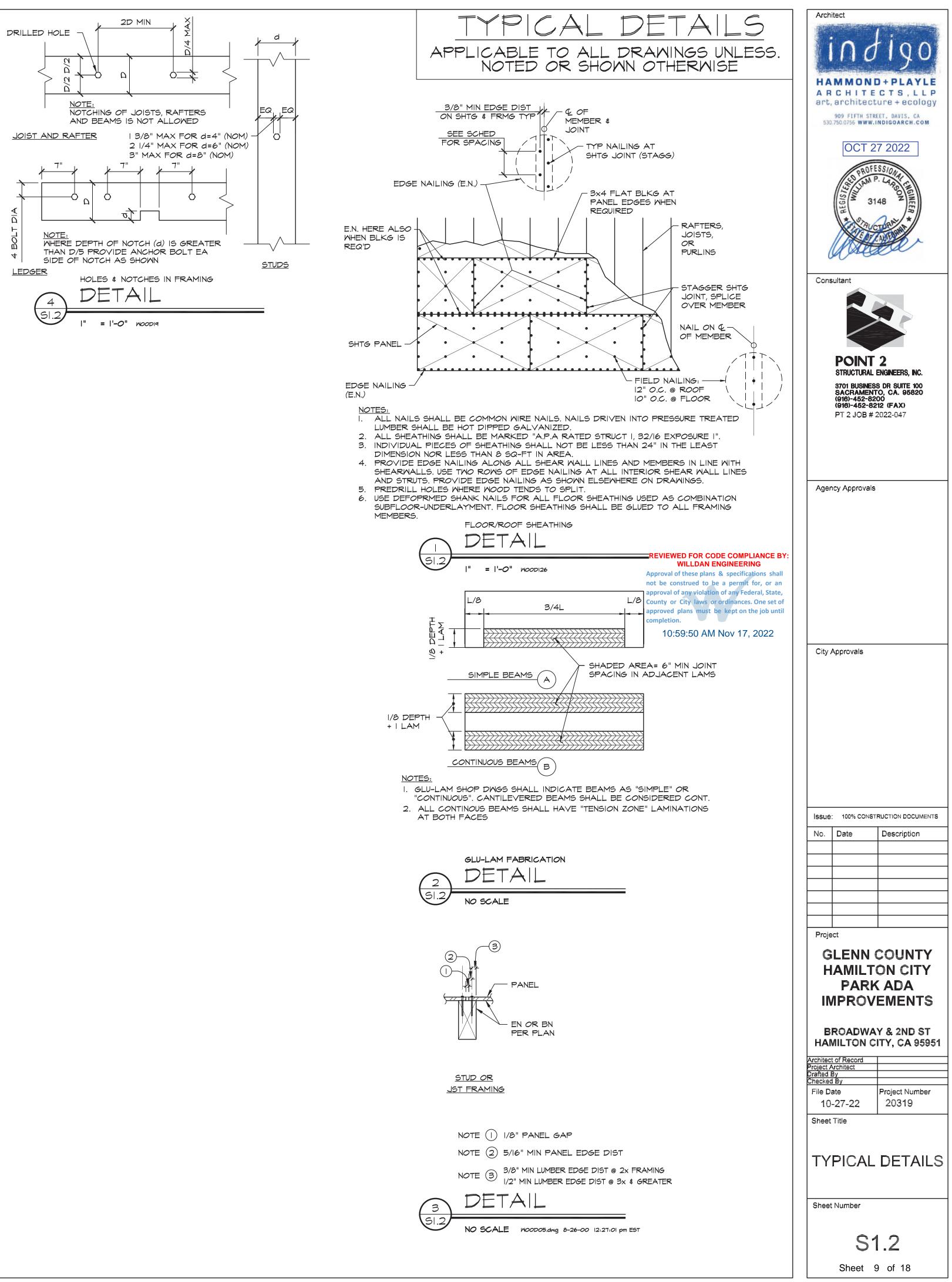
- OPTIONS.
- COMMON, BOX OR CASING.
- E. WHERE A RAFTER IS FASTENED TO AN ADJACENT PARALLEL CEILING JOIST IN PLATE IN ACCORDANCE WITH THIS SCHEDULE, THE NUMBER OF TOENAILS IN THE RAFTER SHALL BE PERMITTED TO BE REDUCED BY ONE NAIL.

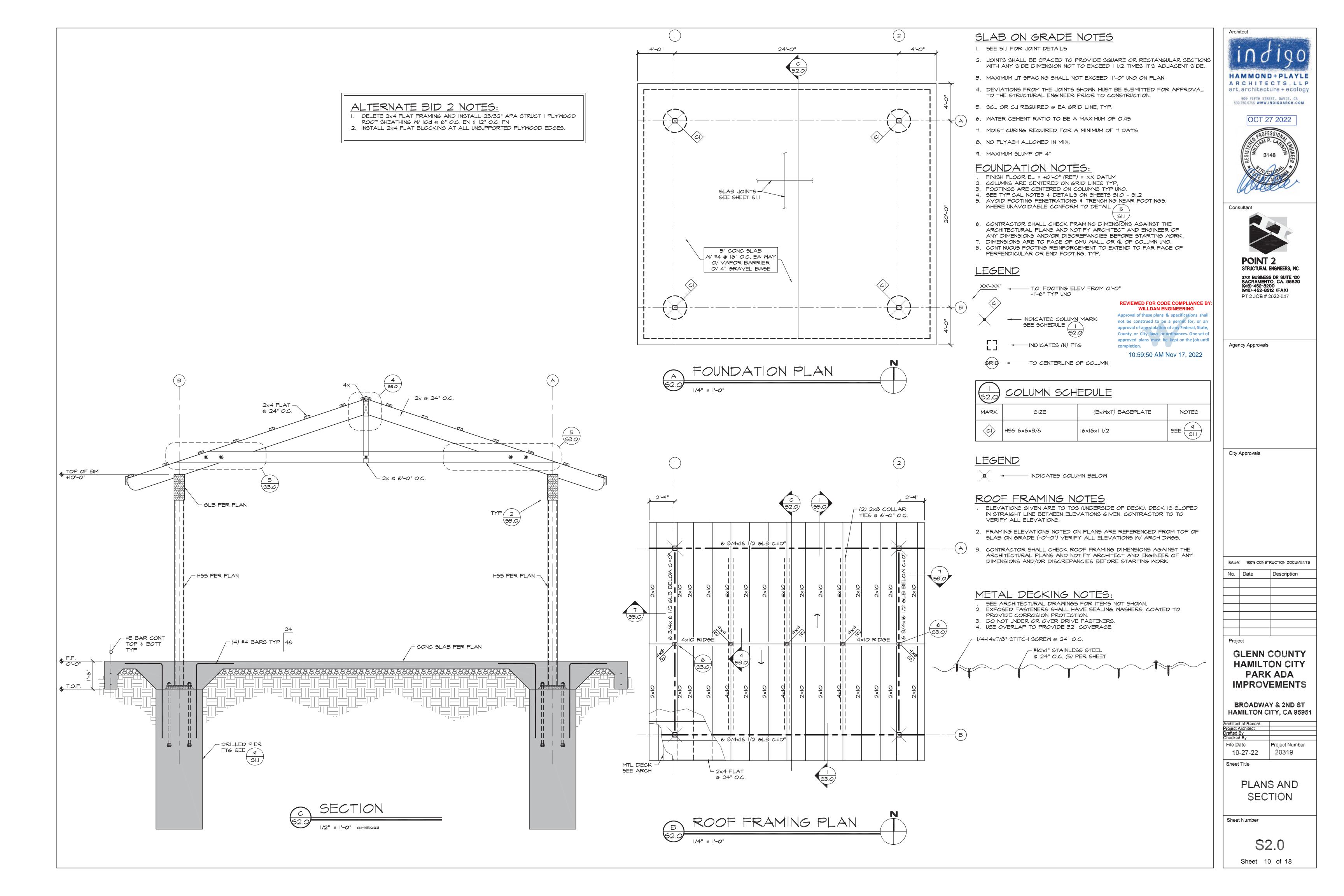
TABLE NO. 2304.10.1 FASTENING SCHEDULE.

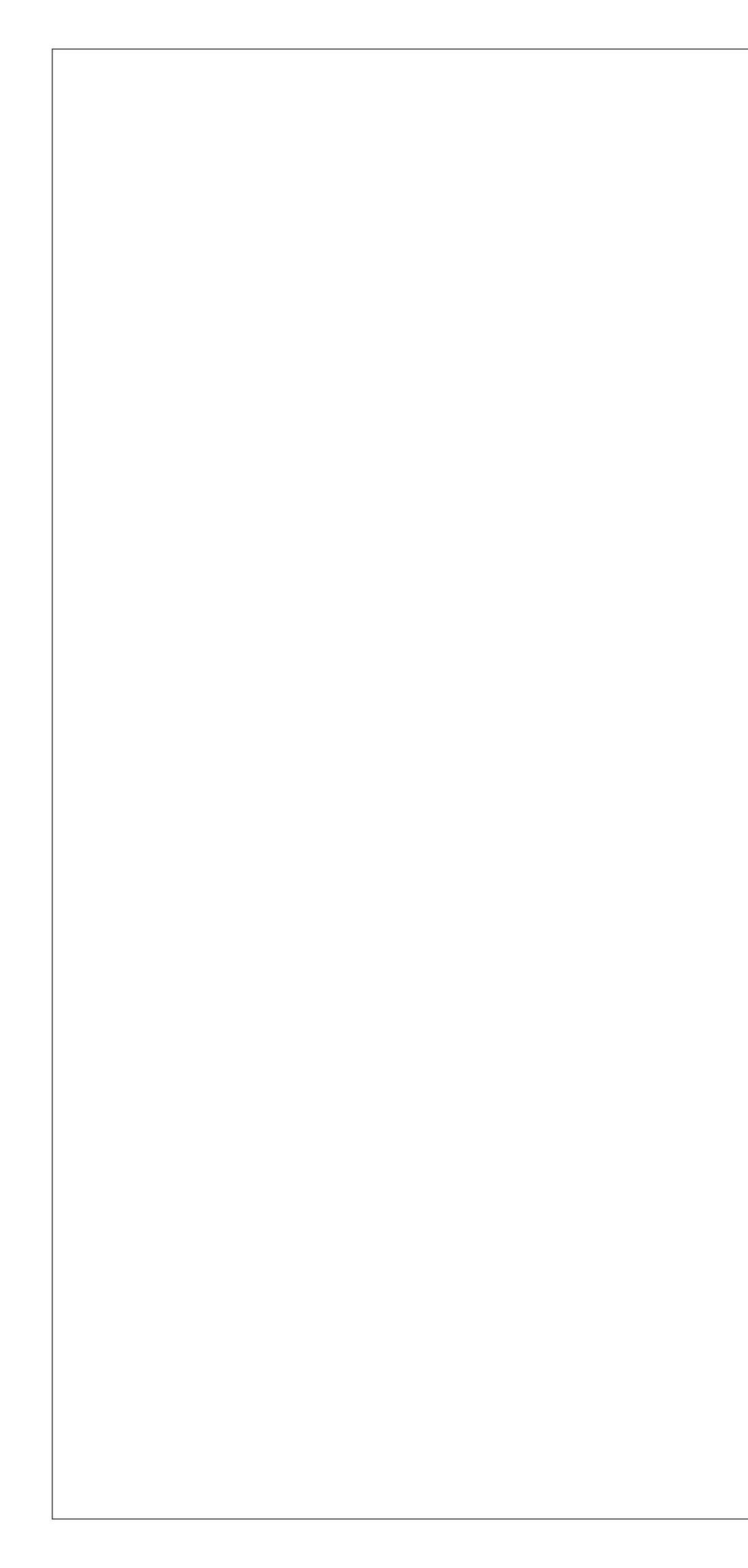
C. NAILS SPACED AT 6" AT INTERMEDIATE SUPPORTS WHERE SPANS ARE 48" OR MORE. FOR NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO SECTION 2305. NAILS FOR WALL SHEATHING ARE PERMITTED TO BE

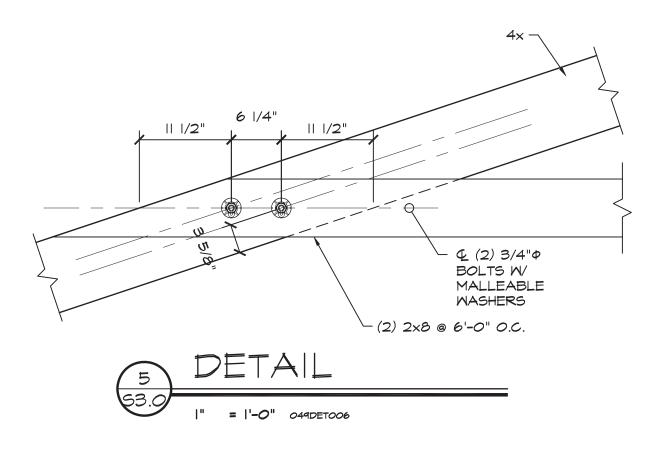
D. SPACING SHALL BE 6" ON CENTER ON THE EDGES AND 12" ON CENTER AT INTERMEDIATE SUPPORTS FOR NONSTRUCTURAL APPLICATIONS. PANEL SUPPORTS AT 16" (20" IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, UNLESS OTHERWISE MARKED).

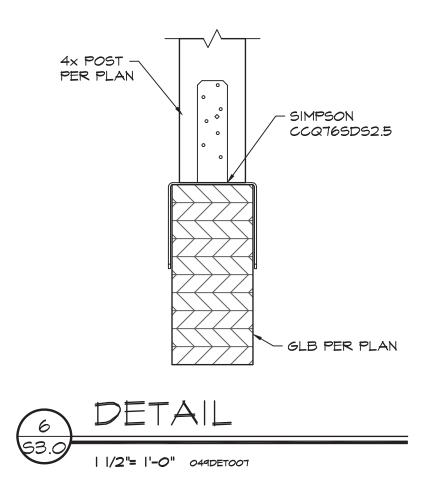
ACCORDANCE WITH THIS SCHEDULE AND THE CEILING JOIST IS FASTENED TO THE TOP

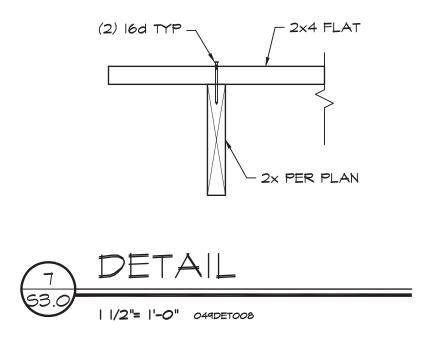




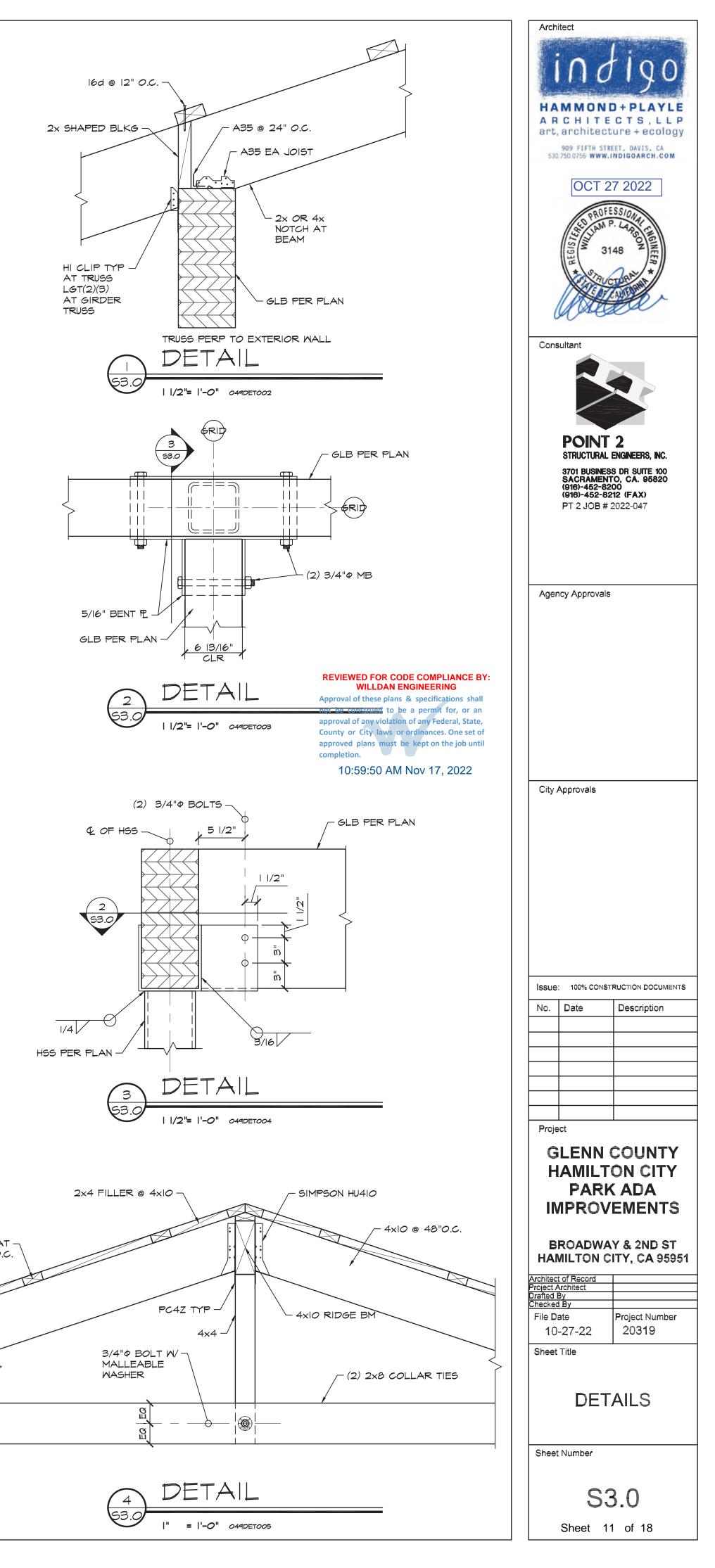


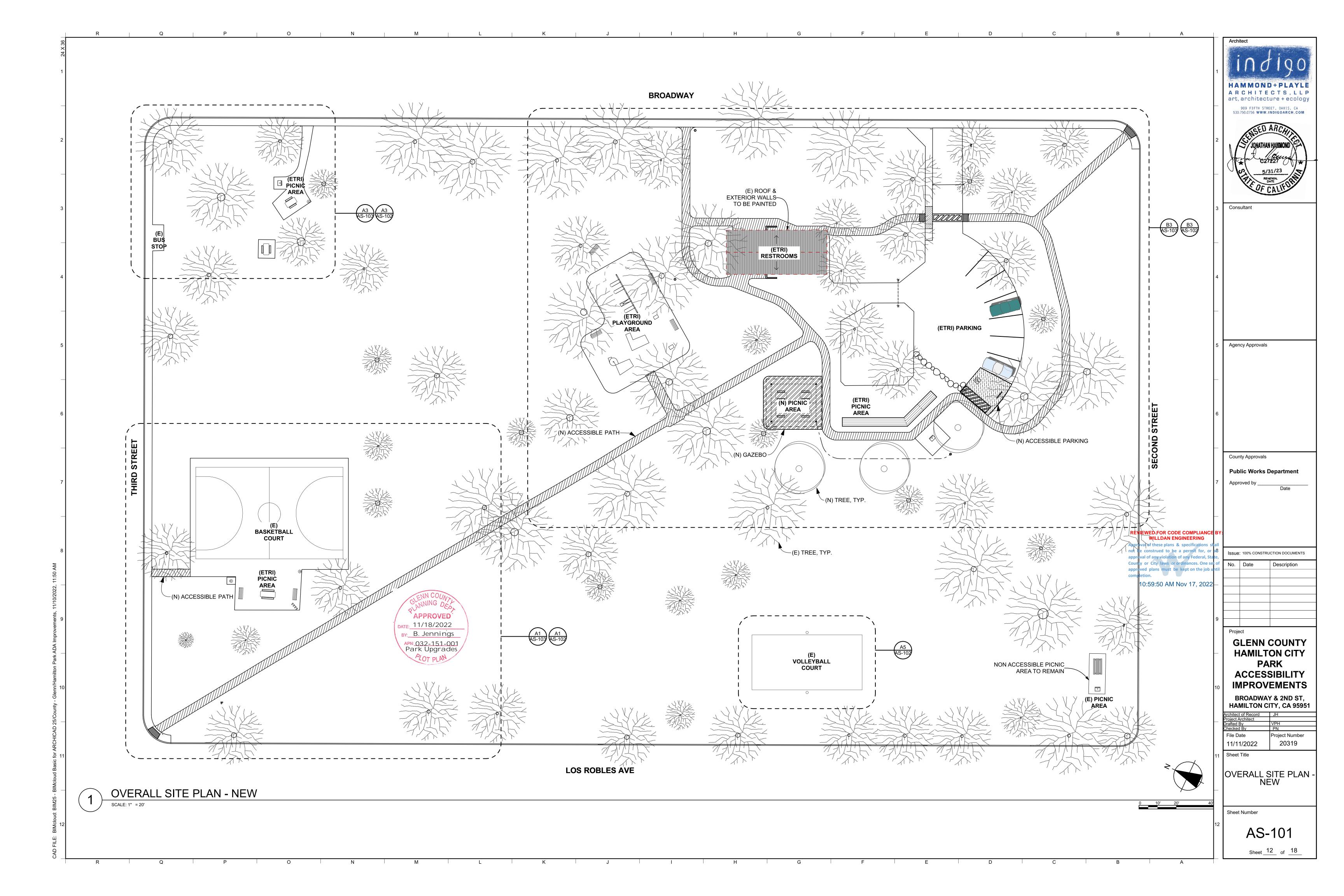


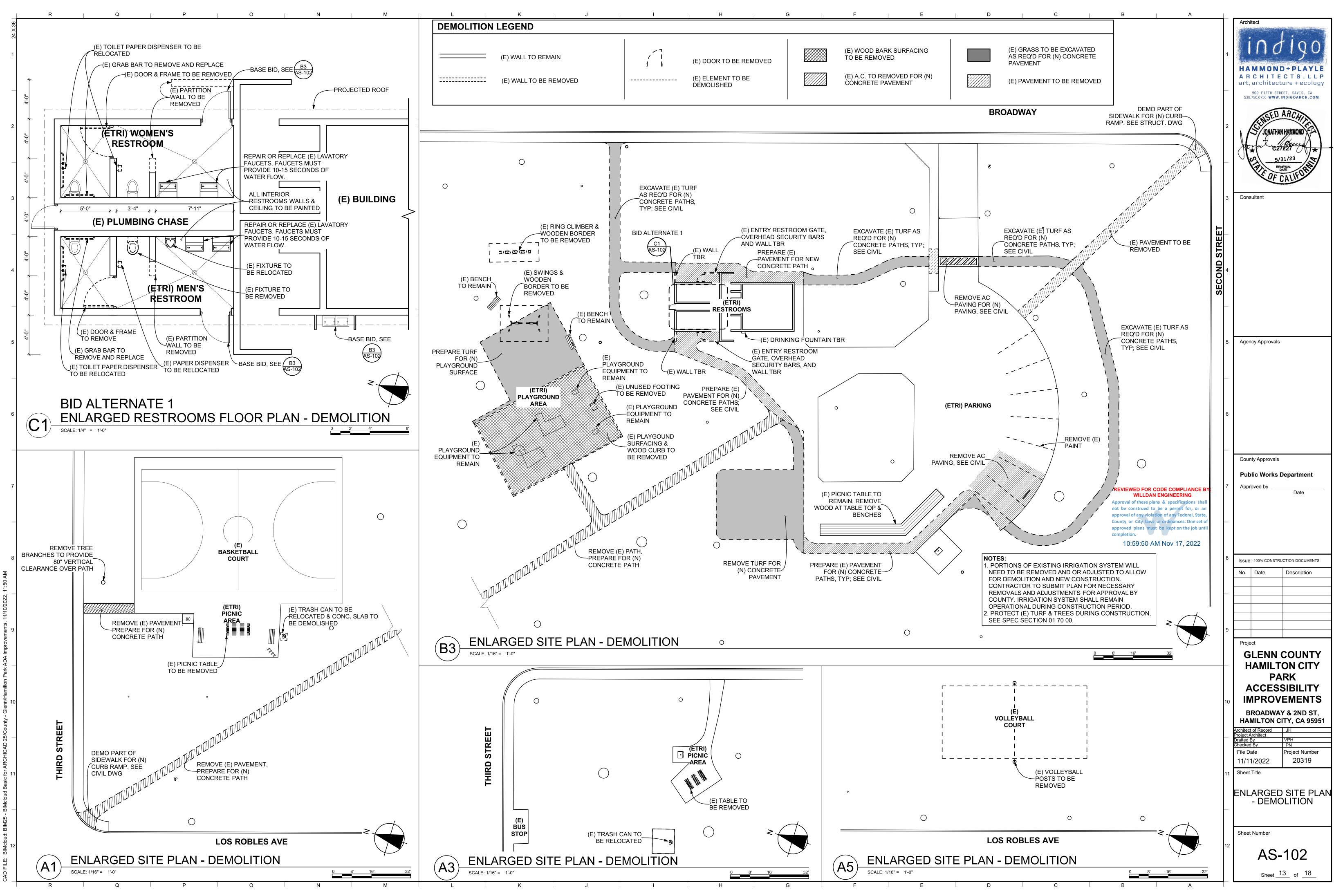


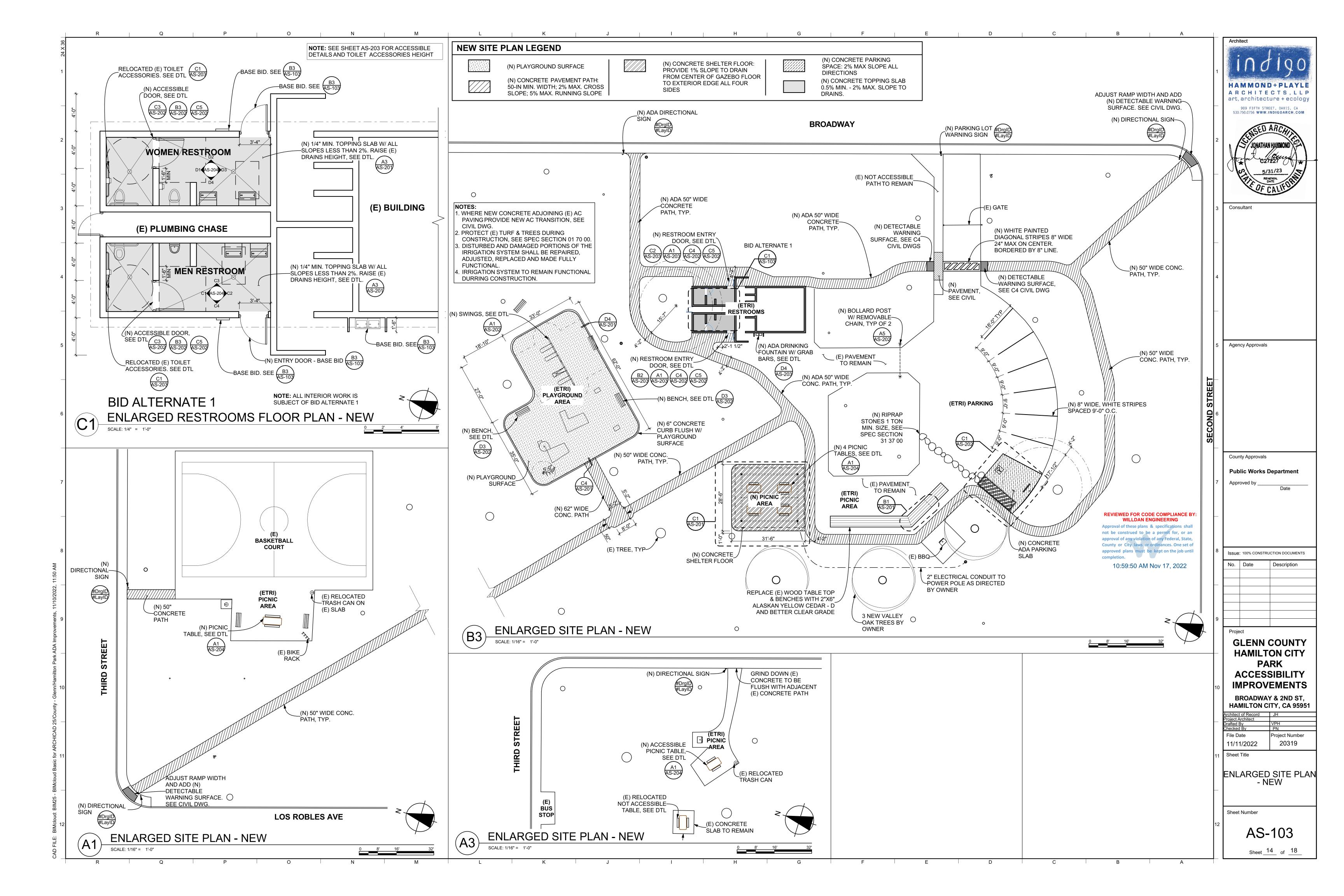


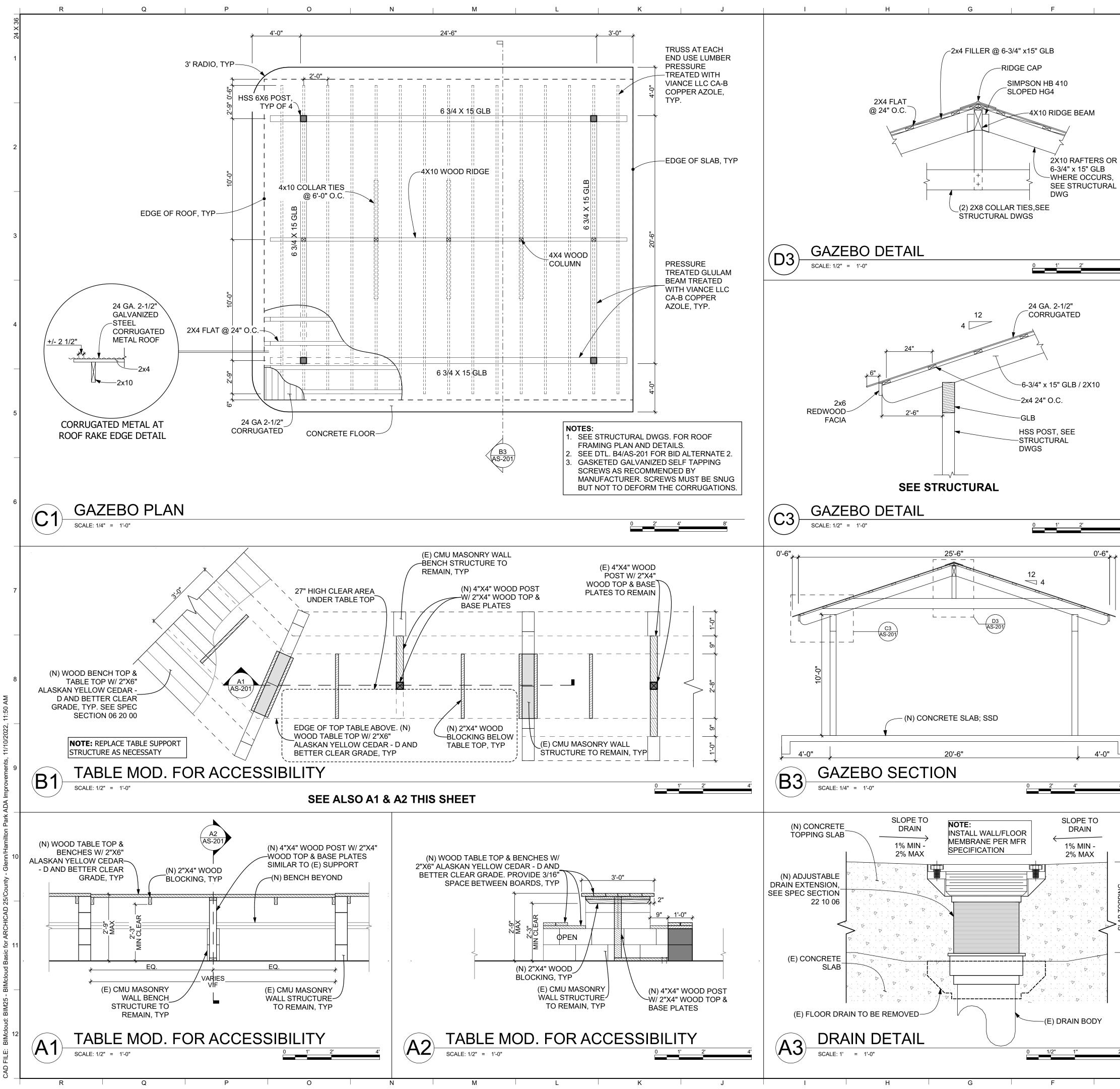
2x4 FLAT -@ 24" *O*.C.











0'-6"**

4'-0"

